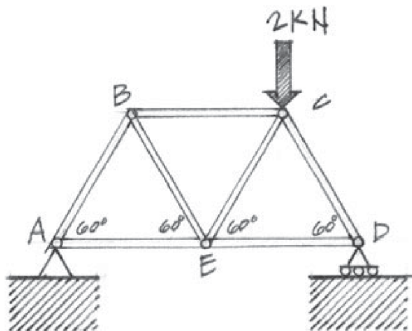


Physics 8 — Monday, November 11, 2019

- ▶ Last week, you skimmed Ch4 (load tracing) and read Ch5 (strength of materials) of Onouye/Kane. This week, you're reading Ch6 (cross-sectional properties) and Ch7 (simple beams).

How many “reaction forces” (or components thereof) are exerted by the supports (i.e. exerted on the truss by the supports)?



(A) 1

(B) 2

(C) 3

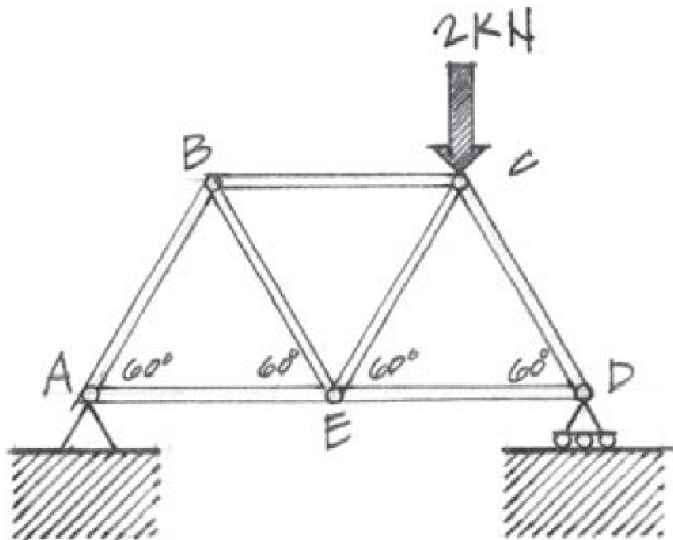
(D) 4

(E) 5

(F) 6

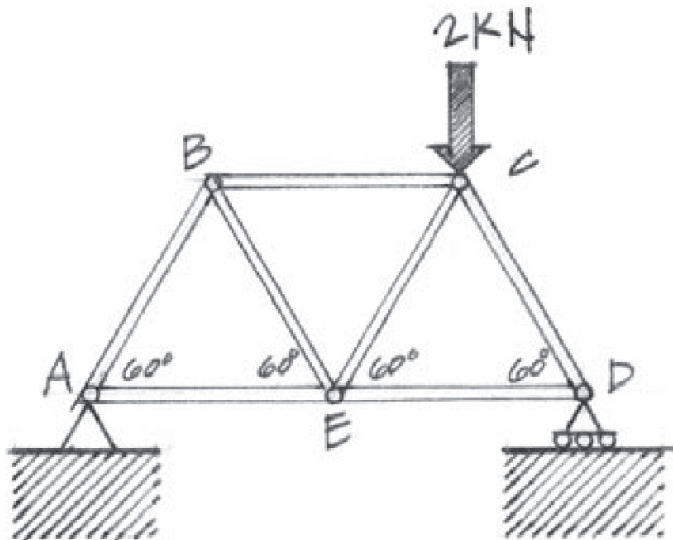
(G) 7

(H) 8



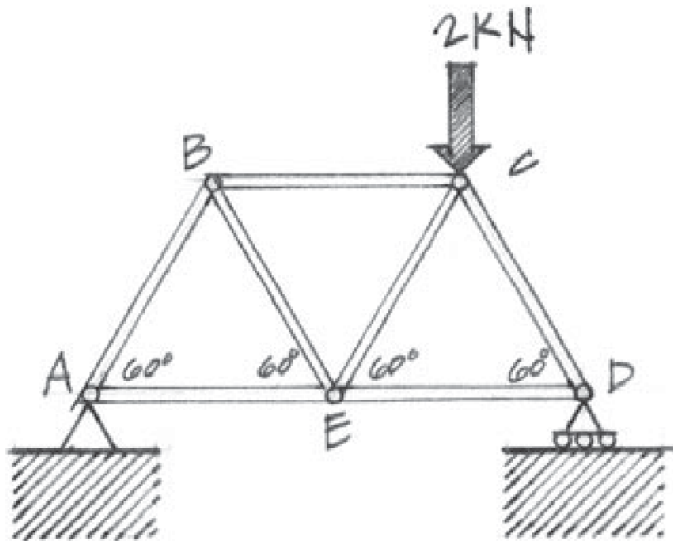
How many "reaction forces" are exerted by the supports (i.e. exerted on the truss by the supports)?

- (A) 1 (B) 2 (C) 3 (D) 4 (E) 5 (F) 6 (G) 7 (H) 8

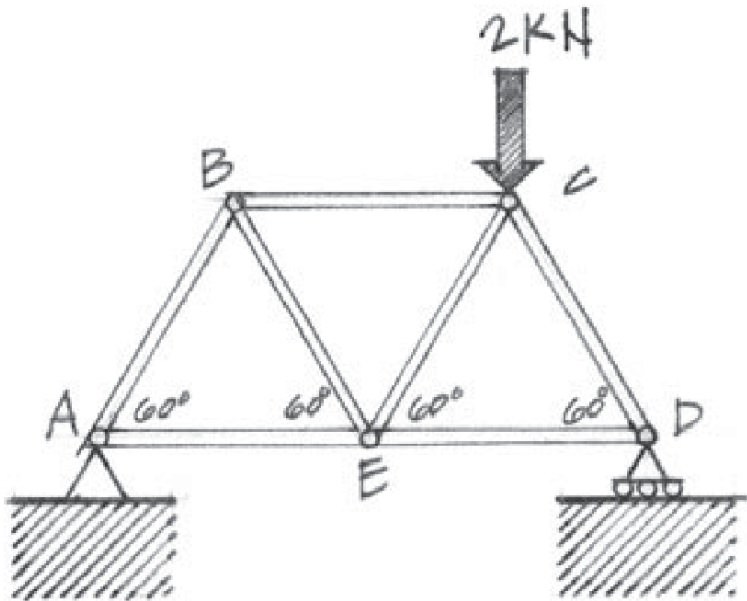


How many internal forces (tensions or compressions in the members) do we need to solve for to “solve” this truss?

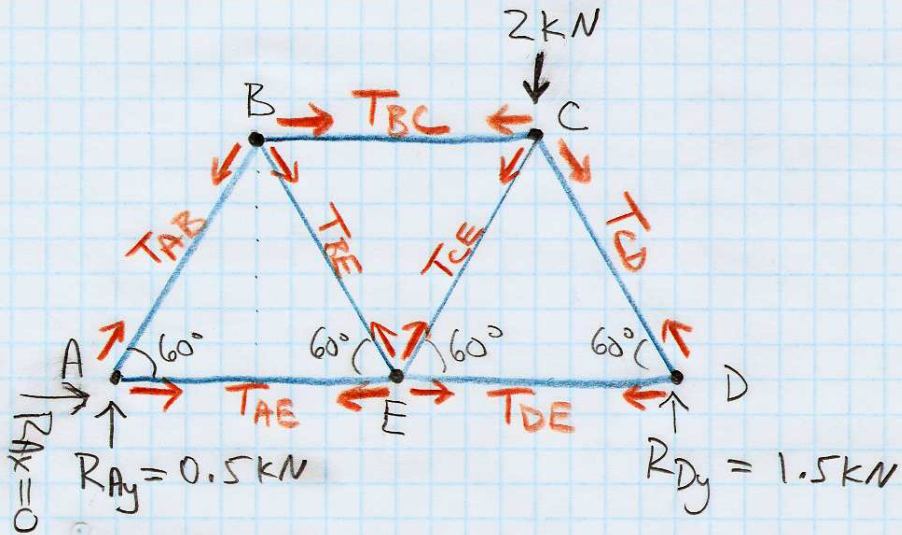
- (A) 1 (B) 2 (C) 3 (D) 4 (E) 5 (F) 6 (G) 7 (H) 8



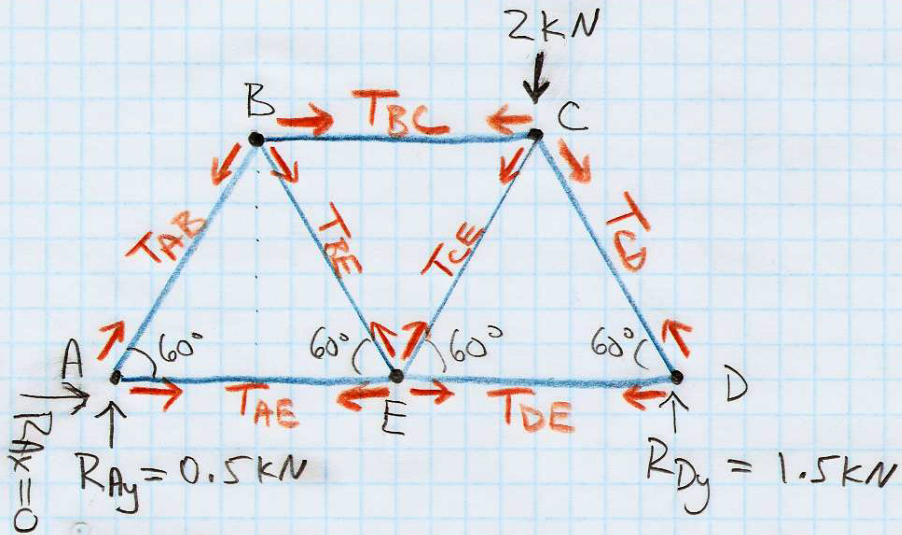
Do you see any joint at which there are ≤ 2 unknown forces (in this context, total, internal+external)? If so, we can start there. If not, we need to start with an EFD for the truss as a whole.



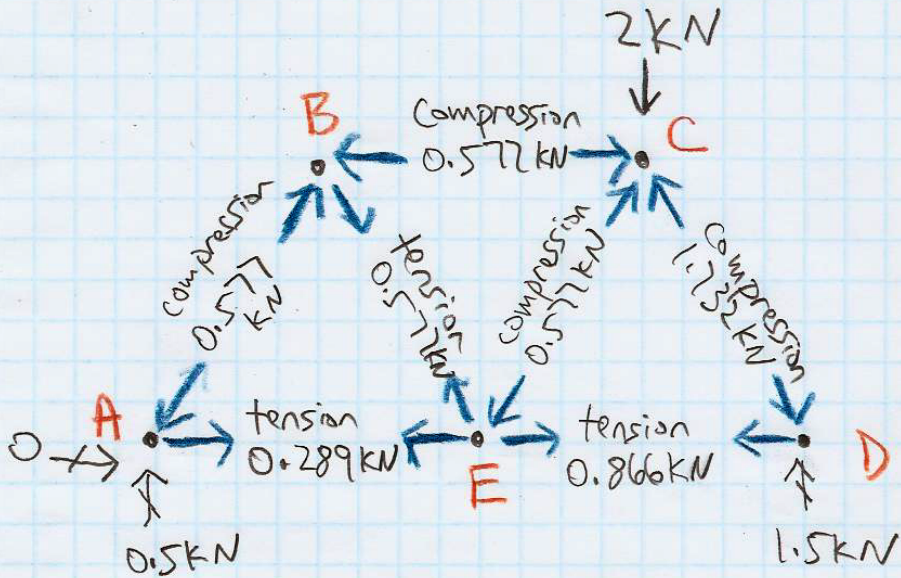
Try to guess $R_{A,x}$, $R_{A,y}$, and $R_{D,y}$ by inspection. Then let's check with the usual equations.



Now start from a joint having ≤ 2 unknown forces. In this case, I just went through the joints alphabetically. You can make your life easier by seeking out equations having just 1 unknown.



$T_{AB} = -0.577 \text{ kN}$, $T_{AE} = +0.289 \text{ kN}$, $T_{BE} = +0.577 \text{ kN}$,
 $T_{BC} = -0.577 \text{ kN}$, $T_{CE} = -0.577 \text{ kN}$, $T_{CD} = -1.732 \text{ kN}$,
 $T_{DE} = +0.866 \text{ kN}$. My notation: tension > 0 , compression < 0 .



$$\cos 60^\circ = 0.5, \quad \sin 60^\circ = 0.866$$

$$\text{Joint A: } 0 + T_{AE} + T_{AB} \cos 60^\circ = 0$$

$$\left(\begin{array}{l} 0.5 \text{ kN} + T_{AB} \sin 60^\circ = 0 \Rightarrow T_{AB} = -0.577 \text{ kN} \\ T_{AE} = +0.289 \text{ kN} \end{array} \right.$$

$$\text{Joint B: } -T_{AB} \cos 60^\circ + T_{BE} \cos 60^\circ + T_{BC} = 0$$

$$\left(\begin{array}{l} -T_{AB} \sin 60^\circ - T_{BE} \sin 60^\circ = 0 \Rightarrow T_{BE} = +0.577 \text{ kN} \\ T_{BC} = -0.577 \text{ kN} \end{array} \right.$$

$$\text{Joint C: } -T_{BC} - T_{CE} \cos 60^\circ + T_{CD} \cos 60^\circ = 0 \Rightarrow T_{CD} = T_{CE} - 1.155 \text{ kN}$$

(could do
CD here at
this point)

$$-2 \text{ kN} - T_{CE} \sin 60^\circ - T_{CD} \sin 60^\circ = 0$$

$$-2 \text{ kN} - T_{CE} \sin 60^\circ - (T_{CE} - 1.155 \text{ kN}) \sin 60^\circ = 0$$

$$2 T_{CE} \sin 60^\circ = -2 \text{ kN} + 1.0 \text{ kN} = -1 \text{ kN}$$

$$T_{CE} = -0.577 \text{ kN}$$

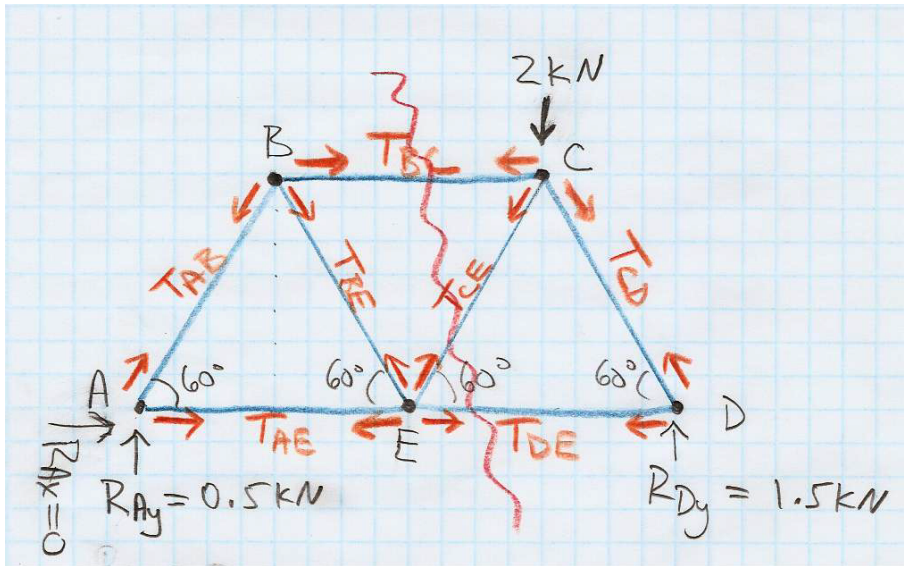
$$\Rightarrow T_{CD} = T_{CE} - 1.155 \text{ kN} = -1.732 \text{ kN} = T_{CD}$$

$$\text{Joint D: } -T_{DE} - T_{CD} \cos 60^\circ = 0 \Rightarrow T_{DE} = +0.866 \text{ kN}$$

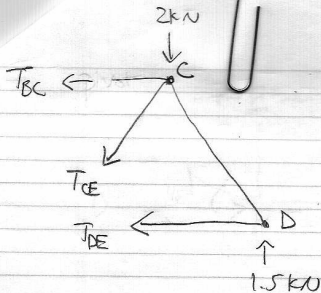
$$\text{(check): } 1.5 \text{ kN} + T_{CD} \sin 60^\circ = 0 \quad \checkmark$$

$$\text{(check Joint E): } -T_{AE} + T_{DE} - T_{BE} \cos 60^\circ + T_{CE} \cos 60^\circ = 0 \quad \checkmark$$

$$+T_{BE} \sin 60^\circ + T_{CE} \sin 60^\circ = 0 \quad \checkmark$$



Let's try drawing an EFB for the **right** side of the cut ("section").
 (We'll start next time with the Method of Sections for this truss.)



$$\sum F_x = 0 \Rightarrow -T_{BC} - T_{CE} \cos 60^\circ - T_{DE} = 0$$

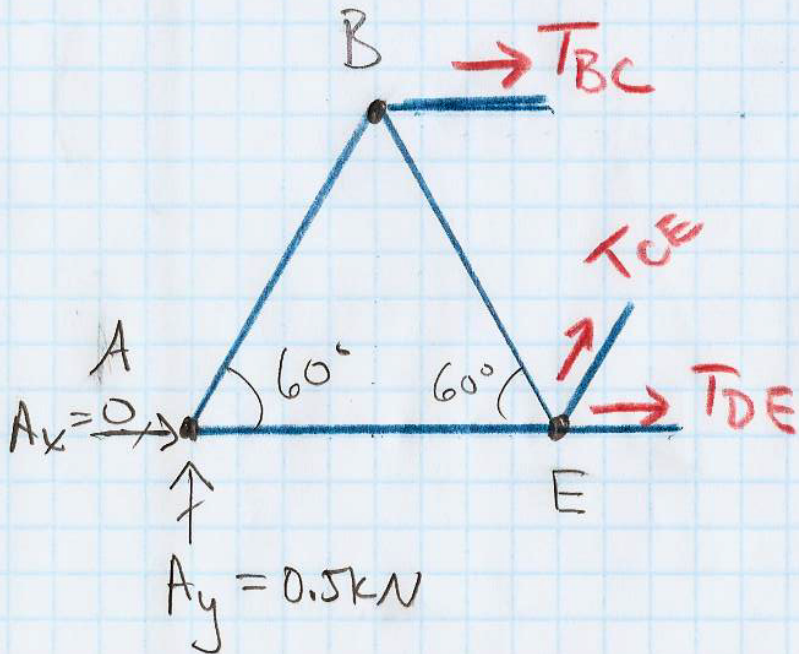
$$\sum F_y = 0 \Rightarrow -2 \text{ kN} + 1.5 \text{ kN} - T_{CE} \sin 60^\circ = 0$$

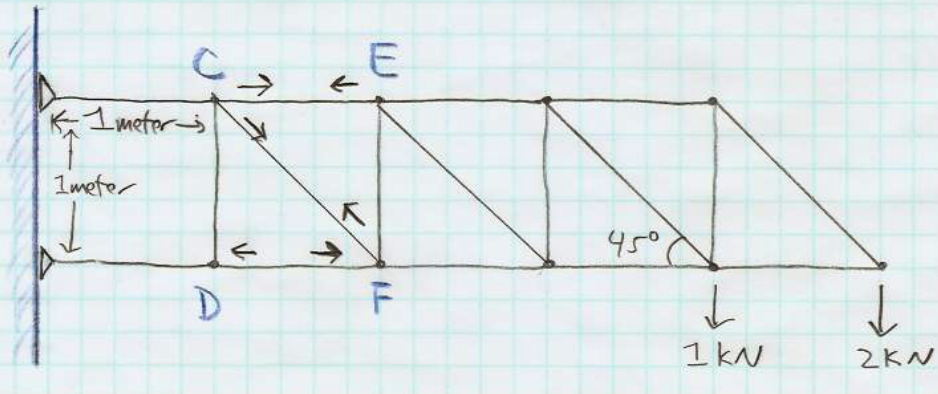
$$\sum M_C = 0 \Rightarrow +(1.5 \text{ kN}) \left(\frac{L}{2} \right) - T_{DE} L \sin 60^\circ = 0$$

$$\rightarrow T_{DE} = \frac{1.5 \text{ kN}}{2 \sin 60^\circ} = +0.866 \text{ kN}$$

$$\rightarrow T_{CE} = \frac{-0.5 \text{ kN}}{\sin 60^\circ} = -0.577 \text{ kN}$$

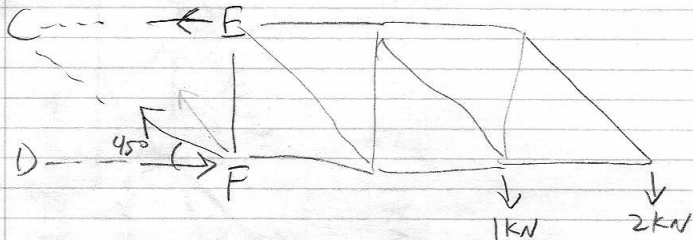
$$\rightarrow T_{BC} = -(T_{CE} \cos 60^\circ + T_{DE}) = -0.577$$





Here's another truss problem that you can solve using the Method of Sections. Find forces in members **CE**, **CF**, and **DF**, with assumed force directions as shown.

- ▶ What happens if an assumed force direction is backwards?
- ▶ Where should we "section" the truss?
- ▶ Then what do we do next?



$$\sum F_x = 0 = -CE - CF \cos 45^\circ + DF = 0$$

$$\sum F_y = 0 = CF \sin 45^\circ = 3 \text{ kN} \Rightarrow \boxed{CF = 3 \text{ kN} \sqrt{2}}$$

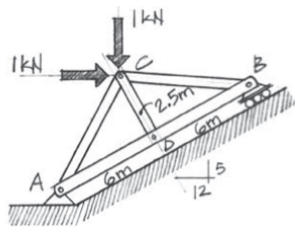
$$\sum M_{DF} = 0 = CE(1) - (1 \text{ kN})(2) - (2 \text{ kN})(3)$$

$$\Rightarrow \boxed{CE = 8 \text{ kN}}$$

$$DF = CE + CF \cos 45^\circ = 8 \text{ kN} + 3 \text{ kN}$$

$$\boxed{DF = 11 \text{ kN}}$$

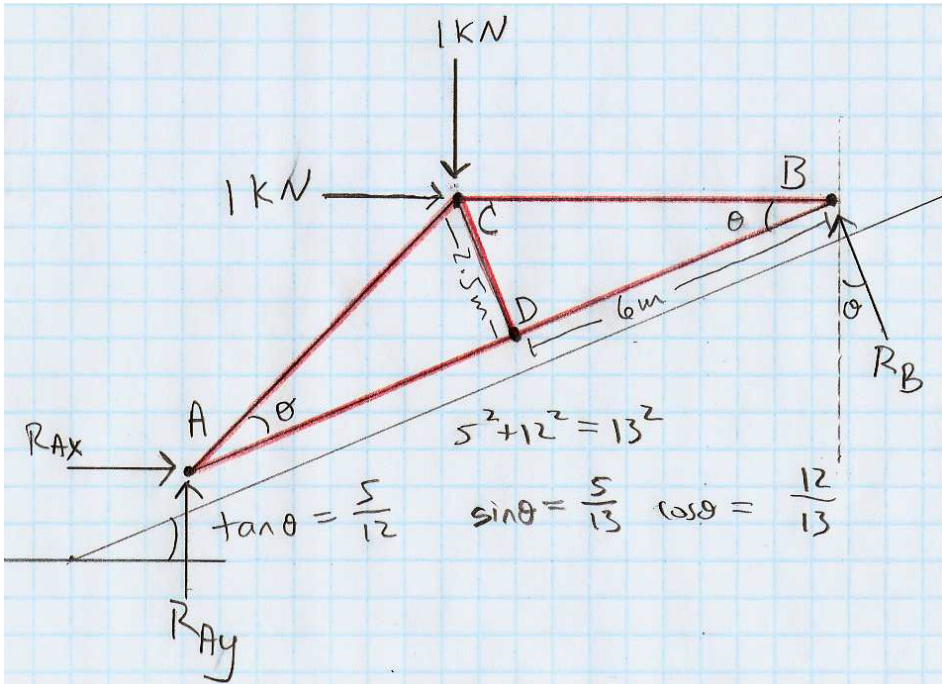
2.38 An inclined king-post truss supports a vertical and horizontal force at C. Determine the support reactions developed at A and B.



This is not really a “truss problem,” since we’re not asked to solve for the internal forces in the truss, but it is an example of a pretty tricky equilibrium problem.

Let’s try working through this together in class. (I think it’s deviously tricky!)

Notice, from the given dimensions: the angle of the incline is the same as the interior angle at joints A and B of the truss. Also notice pin/hinge support at A and roller support at B.



$$\textcircled{1} \quad 0 = \sum_{\text{on truss}} F_x = R_{Ax} + 1 \text{ kN} - R_B \sin \theta$$

$$\Rightarrow R_{Ax} = R_B \sin \theta - 1 \text{ kN}$$

$$\textcircled{2} \quad 0 = \sum_{\text{on truss}} F_y = R_{Ay} - 1 \text{ kN} + R_B \cos \theta$$

$$\Rightarrow R_{Ay} = 1 \text{ kN} - R_B \cos \theta$$

$$\textcircled{3} \quad 0 = \sum M_A = R_B (12 \text{ m}) - 1 \text{ kN} (12 \text{ m} \sin \theta) - 1 \text{ kN} (6.5 \text{ m} \cos 2\theta)$$

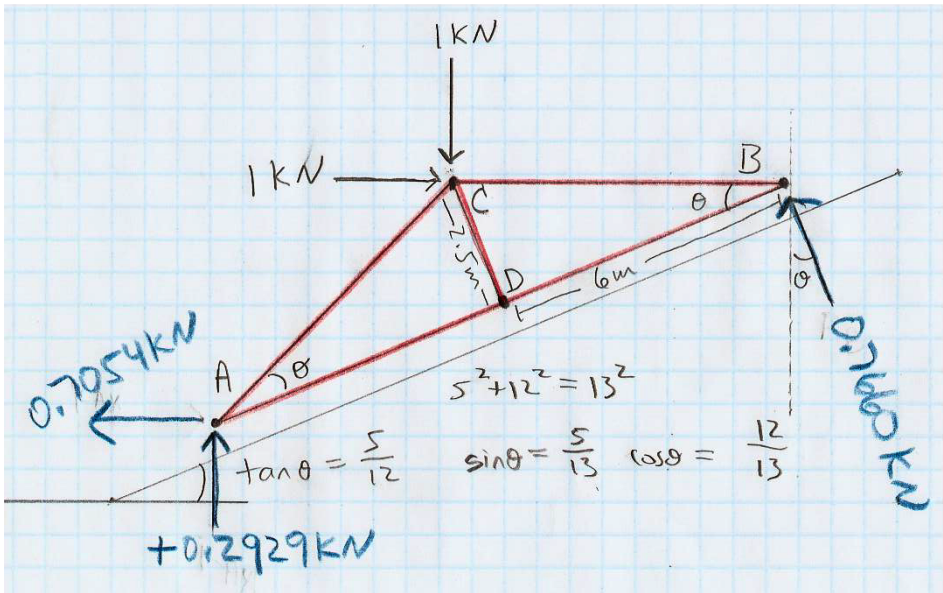
$$\Rightarrow R_B = 1 \text{ kN} (\sin \theta) + \frac{6.5}{12} \text{ kN} (\cos 2\theta) = 0.7660 \text{ kN}$$

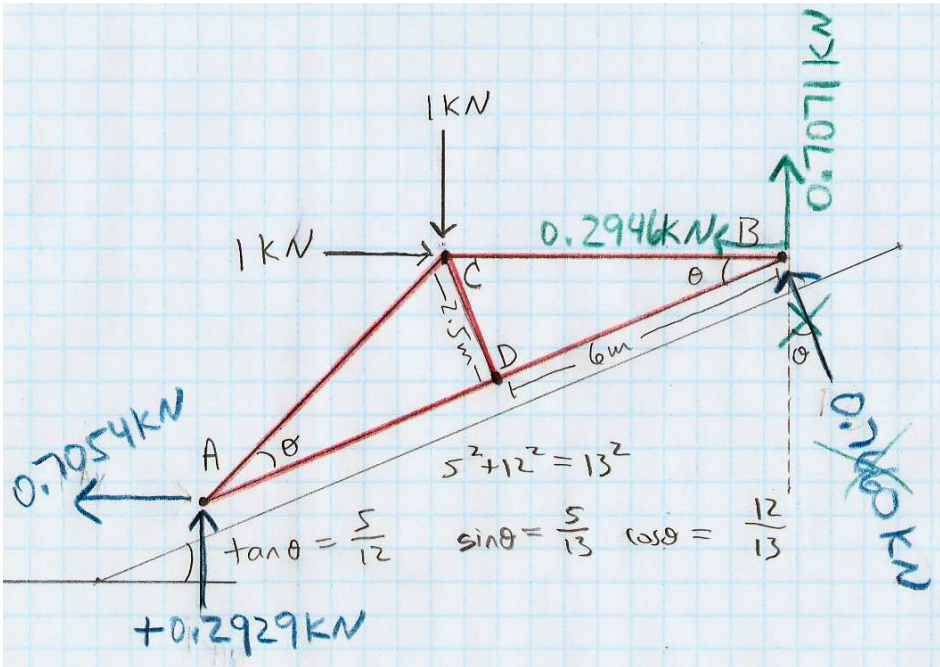
$$(R_B)_x = -R_B \sin \theta = -0.2946 \text{ kN}$$

$$(R_B)_y = R_B \cos \theta = 0.7071 \text{ kN}$$

$$R_{Ax} = (0.2946 - 1) \text{ kN} = -0.7054 \text{ kN}$$

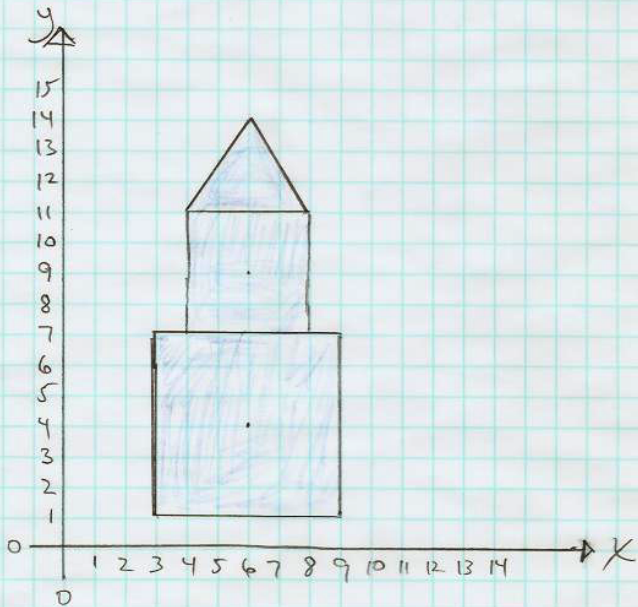
$$R_{Ay} = (1 - 0.7071) \text{ kN} = 0.2929 \text{ kN}$$





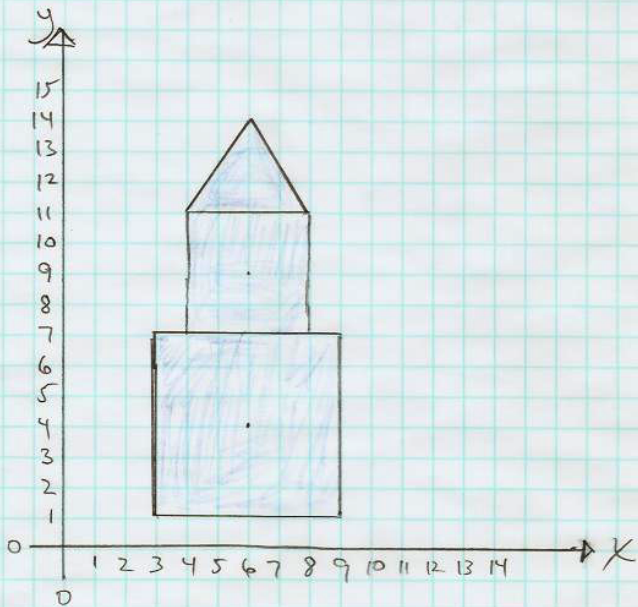
- ▶ The idea of computing centroids of simple and composite shapes is very, very briefly introduced in O/K ch3 (in the context of “distributed loads”), and is discussed in much more detail in O/K ch6 (you read this week, for Monday).
- ▶ Let’s go through one example using rectangles and triangles. It will help you in cases when you need to solve for the “reaction forces” on a beam that carries distributed loads. (Example coming up next.)

What is X_{centroid} for the shaded area?



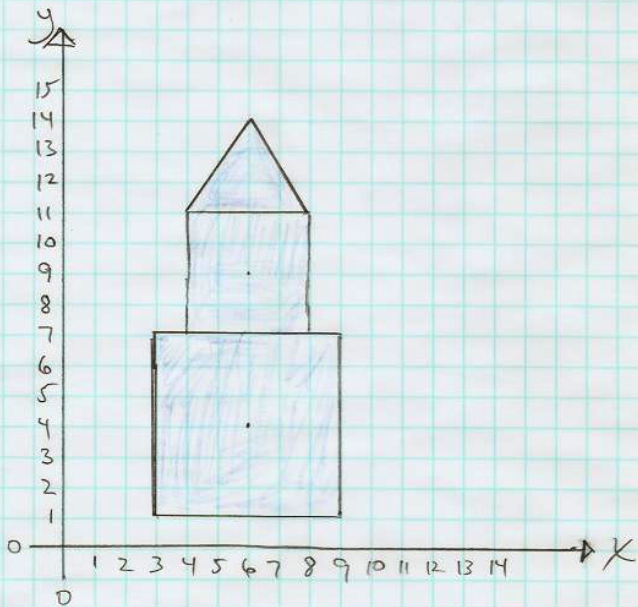
- (A) 0
- (B) 3
- (C) 6
- (D) 9

What are the areas of the three individual polygons?



- (A) 36, 16, 16
- (B) 36, 16, 12
- (C) 36, 16, 8
- (D) 36, 16, 6

What are the Y_{centroid} values of the three individual polygons?



- (A) 4, 9, 11
- (B) 4, 9, 11.667
- (C) 4, 9, 12
- (D) 4, 9, 12.333
- (E) 4, 9, 12.5
- (F) 4, 9, 13
- (G) 4, 9, 14

What is Y_{centroid} for the whole shaded area?

(A)

$$\frac{4 + 9 + 12}{3} = 8.33$$

(B)

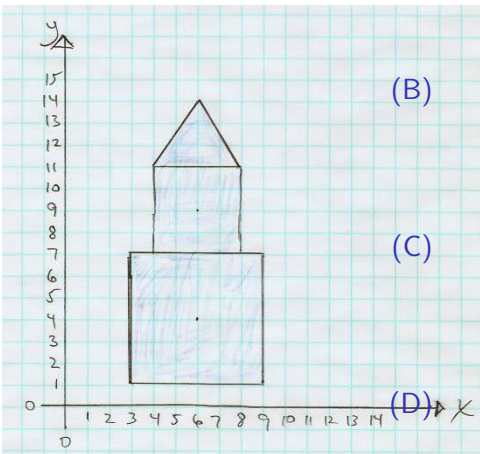
$$\frac{(4)(36) + (9)(16) + (12)(6)}{36 + 16 + 6} = 6.21$$

(C)

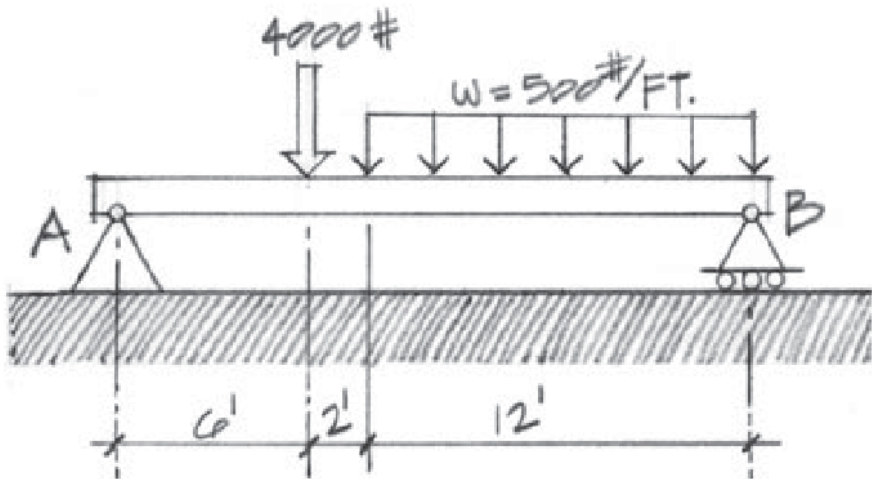
$$\frac{(4)(36) + (9)(16) + (12)(6)}{4 + 9 + 12} = 14.4$$

(D)

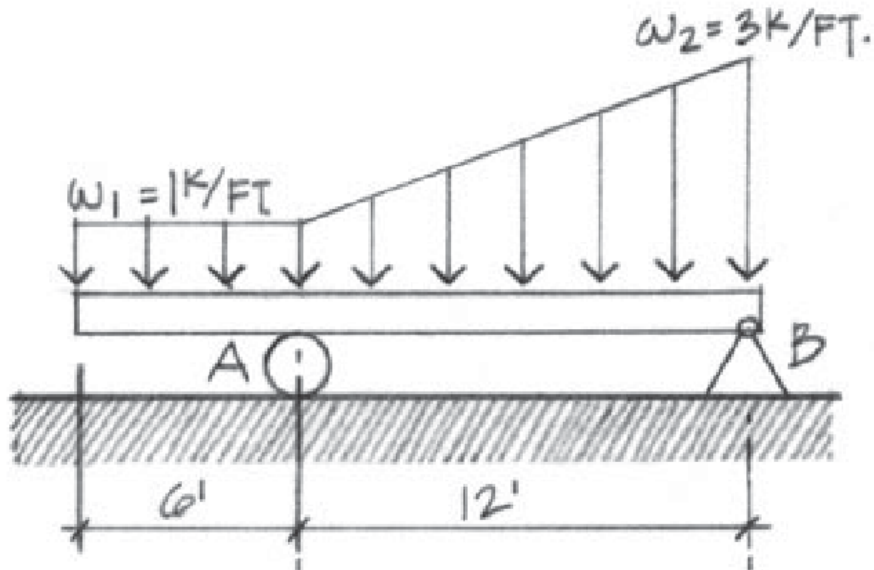
$$\frac{(4^2)(36) + (9^2)(16) + (12^2)(6)}{36 + 16 + 6} = 47.2$$



There's one problem similar to this (but using metric units) on HW(?): Determine the support reactions at A and B.



This one is harder than your homework, because the distributed load is non-uniform: Determine the support reactions at A and B.



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- ▶ Last week, you skimmed Ch4 (load tracing) and read Ch5 (strength of materials) of Onouye/Kane. This week, you're reading Ch6 (cross-sectional properties) and Ch7 (simple beams).